

ABBREVIATIONS

Table with 4 columns: ABBR., DESCRIPTION, ABBR., DESCRIPTION. Lists various construction abbreviations such as AB (Anchor Bolt), ACI (American Concrete Institute), AISI (American Iron and Steel Institute), etc.

DESIGN CRITERIA

- A. THE CONSTRUCTION DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE 2018 VIRGINIA UNIFORM STATEWIDE BUILDING CODE.
B. DESIGN LOAD DATA:
a. DEAD LOADS (IN ADDITION TO SELF-WEIGHT):
1. ROOFS: 12 PSF
b. LIVE LOADS:
1. FLOORS: 100 PSF
2. ROOFS: 30 PSF
c. SNOW DESIGN DATA:
1. GROUND SNOW LOAD (Pg): 42.5 PSF
2. FLAT-ROOF SNOW LOAD (Pfl): 30 PSF
3. SNOW EXPOSURE FACTOR (Ce): 1.0
4. THERMAL FACTOR (Ct): 1.0
d. WIND DESIGN DATA:
1. BASIC WIND SPEED
2. (Vws): 90 MPH
3. WIND IMPORTANCE FACTOR (Iw): 1.00
4. BUILDING RISK CATEGORY: CATEGORY II
5. WIND EXPOSURE CATEGORY: EXPOSURE-C
INTERNAL PRESSURE COEFFICIENT: +/-0.18
e. WOOD TRUSSES:
1. TOP CHORD: LOADS AS INDICATED ABOVE
2. BOTTOM CHORD: 5 PSF DEAD LOAD
f. SOIL BEARING PRESSURE: 1,500 PSF PRESUMPTIVE
C. NO PROVISIONS FOR FUTURE EXPANSIONS WERE MADE IN THE STRUCTURAL DESIGN.

GENERAL STRUCTURAL NOTES

- A. ALL WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE 2018 VIRGINIA UNIFORM STATEWIDE BUILDING CODE.
a. NEW CONSTRUCTION SHALL COMPLY WITH THE VIRGINIA CONSTRUCTION CODE.
B. STRUCTURAL DRAWINGS DO NOT SHOW ALL OPENINGS. COORDINATE WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS.
C. ALL SECTIONS AND DETAILS SHOWN SHALL BE CONSIDERED TYPICAL AND SHALL APPLY AT SIMILAR CONDITIONS/LOCATIONS.
D. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SHALL COORDINATE ALL STRUCTURAL PLANS AND DETAILS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS BEFORE THE START OF WORK. VERIFY THE REQUIREMENTS OF OTHER TRADES FOR INSERTS, CLIPS, HANGERS, SUPPORTS, ETC. TO BE INTEGRATED WITH STRUCTURAL COMPONENTS. NOTIFY ENGINEER/ARCHITECT OF DEFICIENCIES PRIOR TO THE START OF CONSTRUCTION.

FOUNDATIONS

- A. REMOVE THE FOLLOWING MATERIALS FROM 5' BEYOND THE FACE OF THE BUILDING: VEGETATION AND TOPSOIL, WALKS AND PAVING, REMAINING PORTIONS OF EXISTING STRUCTURES INDICATED TO BE DEMOLISHED, SOILS OF INADEQUATE BEARING QUALITY ETC.
B. PROOF ROLL THE BUILDING PAD SUBGRADE AT THE DIRECTION OF THE GEOTECHNICAL ENGINEER OF RECORD PRIOR TO BEGINNING FOUNDATION WORK.
C. INSTALL FOUNDATIONS A MINIMUM OF 12 INCHES INTO SOUND, ORIGINAL, UNDISTURBED SOIL OR PROPERLY INSTALLED STRUCTURAL (CONTROLLED) FILL, EITHER OF WHICH SHALL HAVE A MINIMUM SAFE BEARING CAPACITY OF 1,500 PSF AS VERIFIED BY THE GEOTECHNICAL ENGINEER OF RECORD.
D. INSTALL STRUCTURAL FILL WHERE REQUIRED UNDER FOOTINGS CONSISTING OF #21A, OR OTHER WELL-GRADED CRUSHED STONE OR GRAVEL-SAND MIXTURES ACCEPTABLE TO THE GEOTECHNICAL ENGINEER.
E. INSTALL STRUCTURAL FILLS UNDER THE DIRECT SUPERVISION OF A LICENSED GEOTECHNICAL ENGINEER. REQUIRE THE GEOTECHNICAL ENGINEER TO INSPECT EXCAVATIONS PRIOR TO PLACEMENT OF FILL AND ENSURE REMOVAL OF ALL UNSUITABLE MATERIAL.
F. THE GEOTECHNICAL ENGINEER SHALL MONITOR THE SITE & EXCAVATION FOR THE OCCURRENCE OF EXPANSIVE CLAY SOILS. WHERE EXPANSIVE CLAY SOILS OCCUR, UNDERCUT AND BACKFILL ALL EXTERIOR FOOTING TRENCHES AT THE DIRECTION OF THE GEOTECHNICAL ENGINEER.
G. ESTABLISH EXTERIOR FOOTING ELEVATIONS SUCH THAT THE BOTTOMS OF FOOTINGS ARE NOT LESS THAN 30" BELOW FINAL FINISH GRADES. ESTABLISH TOPS OF FOOTINGS ON MASONRY COURSING.
H. STEP WALL FOOTINGS AT GRADE CHANGES PER THE TYPICAL DETAIL.
I. STEP FOOTINGS BELOW UTILITIES PENETRATING FOUNDATION WALLS PER THE TYPICAL DETAIL.
J. SEQUENCE WORK TO ALLOW THE GEOTECHNICAL ENGINEER TO EXAMINE AND APPROVE ALL FOUNDATION BEARING SURFACES PRIOR TO INSTALLATION OF FOOTING REINFORCING AND CONCRETE. PROTECT THE BOTTOM OF FOOTING EXCAVATIONS FROM DISTURBANCES AND WETTING PRIOR TO PLACEMENT OF CONCRETE. PLACE CONCRETE IN FOOTING EXCAVATIONS THE SAME DAY THAT THE BEARING SURFACES ARE APPROVED.
K. BUILD UP AREAS UNDER FLOOR SLABS TO THE REQUIRED GRADE USING SELECT STRUCTURAL FILL ACCEPTABLE TO THE GEOTECHNICAL ENGINEER.
L. COMPACT GRANULAR FILL BELOW SLABS ON GRADE WITH VIBRATING COMPACTORS ACCORDING TO THE DIRECTIONS OF THE GEOTECHNICAL ENGINEER, BUT NOT LESS THAN 95% OF MAXIMUM DENSITY IN ACCORDANCE WITH ASTM D698.

MASONRY

- A. INSTALL CONCRETE MASONRY WORK PER THE REQUIREMENTS OF THE MASONRY SOCIETY TMS 402/602 "BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES".
B. PROVIDE MATERIALS CONFORMING TO THE FOLLOWING:
a. HOLLOW CONCRETE MASONRY UNITS: ASTM C90, 2,000 PSI
b. MORTAR: ASTM C270, TYPE-S UNO
c. GROUT: ASTM C476, 2,000 PSI AT 28 DAYS (MIN). NO ADMIXTURES CONTAINING CHLORIDES ARE PERMITTED.
d. HORIZONTAL JOINT REINFORCING: ASTM A951, MEDIUM DUTY; W2.1 SIDE RODS AND CROSS BARS
1. BELOW GRADE: HOT-DIPPED GALVANIZED PER ASTM A153
2. ABOVE GRADE: MILL GALVANIZED PER ASTM A641
C. PROVIDE STRUCTURAL UNIT MASONRY THAT DEVELOPS NET-AREA COMPRESSIVE STRENGTHS AT 28 DAYS OF Pm = 2,000 PSI. FOR EACH COMBINATION OF MASONRY UNIT TYPE AND MORTAR TYPE, PROVIDE STATEMENT OF AVERAGE NET-AREA COMPRESSIVE STRENGTH OF MASONRY UNITS, MORTAR TYPE, AND RESULTING NET-AREA COMPRESSIVE STRENGTH OF MASONRY DETERMINED ACCORDING TO TMS 602/ ACI 530.1/ASCE 6. DETERMINE NET-AREA COMPRESSIVE STRENGTH OF MASONRY FROM AVERAGE NET-AREA COMPRESSIVE STRENGTHS OF MASONRY UNITS AND MORTAR TYPES (UNIT-STRENGTH METHOD) ACCORDING TO TMS 602/ACI 530.1/ASCE 6.
D. STORE MASONRY UNITS ON ELEVATED PLATFORMS IN A DRY LOCATION. IF UNITS ARE NOT STORED IN AN ENCLOSED LOCATION, COVER TOPS AND SIDES OF STACKS WITH WATERPROOF SHEETING, SECURELY TIED. IF UNITS BECOME WET, DO NOT INSTALL UNTIL THEY ARE DRY.
E. STORE CEMENTITIOUS MATERIALS ON ELEVATED PLATFORMS, UNDER COVER, AND IN A DRY LOCATION. DO NOT USE CEMENTITIOUS MATERIALS THAT HAVE BECOME DAMP.
F. STORE AGGREGATES WHERE GRADING AND OTHER REQUIRED CHARACTERISTICS CAN BE MAINTAINED AND CONTAMINATION AVOIDED.
G. INSTALL CMU OR PRECAST CONCRETE LINTELS OVER ALL OPENINGS GREATER THAN 8 INCHES SPAN AS FOLLOWS:
a. LINTEL WIDTH SAME THICKNESS AS WALL, 8" NOMINAL DEPTH UNO.
b. REINFORCEMENT: (1) #4 BAR TOP AND BOTTOM FOR EACH 4 INCHES OF WALL THICKNESS OR FRACTION THEREOF.
c. PROVIDE 8" MINIMUM LENGTH OF BEARING AT EACH END.
H. GROUT SOLID ALL MASONRY CELLS AND CAVITIES THAT CONTAIN REINFORCING OR THAT ARE INDICATED TO BE GROUTED. ALLOW MASONRY TO SET A MINIMUM OF 24 HOURS PRIOR TO PLACING GROUT. ROD OR VIBRATE GROUT IN PLACE.
I. PROVIDE VERTICAL WALL REINFORCING IN INCREMENTAL LENGTHS WITH ALLOWANCE FOR LAP SPLICES IN ACCORDANCE WITH THE STANDARD DETAIL.
J. DO NOT USE FROZEN MATERIALS OR MATERIALS MIXED OR COATED WITH ICE OR FROST. DO NOT BUILD ON FROZEN SUBSTRATES. REMOVE AND REPLACE UNIT MASONRY DAMAGED BY FROST OR BY FREEZING CONDITIONS. COMPLY WITH COLD-WEATHER CONSTRUCTION REQUIREMENTS CONTAINED IN TMS 602/ACI 530.1/ASCE 6.
K. COMPLY WITH HOT-WEATHER CONSTRUCTION REQUIREMENTS CONTAINED IN TMS 602/ ACI 530.1/ASCE 6.

CONCRETE

- A. COMPLY WITH PROVISIONS OF FOLLOWING CODES, SPECIFICATIONS AND STANDARDS (CURRENT EDITION), EXCEPT WHERE MORE STRINGENT REQUIREMENTS ARE SHOWN OR SPECIFIED:
a. ACI 301 - "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS"
b. ACI 117 - "SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS."
c. ACI 318 - "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE."
d. ACI 304 - "GUIDE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE."
e. CONCRETE REINFORCING STEEL INSTITUTE (CRSI), "MANUAL OF STANDARD PRACTICE"
B. EXTENT OF CONCRETE WORK IS SHOWN ON DRAWINGS, INCLUDING SCHEDULES, NOTES AND DETAILS WHICH SHOW SIZE AND LOCATION OF MEMBERS AND TYPE OF STRUCTURAL CONCRETE TO BE CAST IN PLACE. FURNISH ALL LABOR, MATERIALS, SERVICES, EQUIPMENT AND HARDWARE REQUIRED IN CONJUNCTION WITH OR RELATED TO THE FORMING, DELIVERY AND POURING OF ALL CAST-IN-PLACE CONCRETE WORK.
C. THE CONCRETE SUPPLIER SHALL HAVE A MINIMUM OF FIVE YEARS OF EXPERIENCE IN MANUFACTURING READY-MIXED CONCRETE PRODUCTS COMPLYING WITH ASTM C 94 REQUIREMENTS FOR PRODUCTION FACILITIES AND EQUIPMENT. THE SUPPLIER MUST BE CERTIFIED ACCORDING TO THE NATIONAL READY MIXED CONCRETE ASSOCIATION'S CERTIFICATION OF READY MIXED CONCRETE PRODUCTION FACILITIES.
D. THE CONCRETE CONTRACTOR SHALL HAVE A MINIMUM OF FIVE YEARS OF EXPERIENCE WITH INSTALLATION OF CONCRETE SIMILAR IN MATERIAL, DESIGN AND EXTENT TO THAT INDICATED FOR THIS PROJECT AND WHOSE WORK HAS RESULTED IN CONSTRUCTION WITH A RECORD OF SUCCESSFUL SERVICE PERFORMANCE.
E. AT LEAST 7 DAYS PRIOR TO BEGINNING CONCRETE WORK, THE CONTRACTOR SHALL CONDUCT A PRE-CONSTRUCTIONS MEETING TO REVIEW THE PROPOSED MIX DESIGNS AND TO DISCUSS REQUIRED METHODS AND PROCEDURES TO PRODUCE CONCRETE CONSTRUCTION OF THE REQUIRED QUALITY. ALSO REVIEW REQUIREMENTS FOR SUBMITTALS, STATUS OF COORDINATING WORK AND AVAILABILITY OF MATERIALS. ESTABLISH WORK PROGRESS SCHEDULE AND PROCEDURES FOR MATERIALS INSPECTION, TESTING AND CERTIFICATIONS.
F. PROVIDE CONCRETE CONFORMING TO THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS:
a. FOOTINGS: 3,000 PSI
b. DRILLED PIERS: 4,000 PSI
c. SLABS ON GRADE: 4,000 PSI
G. ACCEPTANCE CRITERIA FOR CONCRETE STRENGTH: A STRENGTH TEST IS DEFINED AS THE AVERAGE STRENGTH OF AT LEAST TWO 6" X 12" CYLINDER BREAKS OR AT LEAST THREE 4" X 8" CYLINDER BREAKS TESTED AT THE STRENGTH AGE REQUIRED BY APPLICABLE ASTM STANDARD FOR THAT CLASS OF CONCRETE. THE STRENGTH LEVEL OF AN INDIVIDUAL CLASS OF CONCRETE SHALL BE CONSIDERED SATISFACTORY WHEN BOTH OF THE FOLLOWING REQUIREMENTS ARE MET:
a. THE AVERAGE OF ALL SETS OF THREE CONSECUTIVE STRENGTH TESTS EQUAL OR EXCEED THE REQUIRED F'C
b. NO INDIVIDUAL STRENGTH TEST FALLS BELOW THE REQUIRED F'C BY MORE THAN 0.1 F'C OR 500 PSI, WHICHEVER IS GREATER.
H. ALL CONCRETE EXPOSED TO WEATHER AND FREEZE/THAW CONDITIONS SHALL BE AIR ENTRAINED 6% +/- .5%.
I. NO ADMIXTURES OR PRODUCTS CONTAINING CHLORIDES ARE PERMITTED. CONTRACTOR SHALL BE RESPONSIBLE FOR SELECTING ADMIXTURES AND SURFACE TREATMENTS THAT ARE COMPATIBLE WITH THE INTENDED USE OF THE CONCRETE INCLUDING ALL FINAL SURFACE TREATMENTS CALLED FOR WITHIN THIS OR OTHER SPECIFICATIONS OR ON THE CONTRACT DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR FOLLOWING THE MANUFACTURER'S INSTRUCTIONS FOR THE USE OF THEIR PRODUCT INCLUDING ABIDING BY ANY LIMITATIONS PLACED BY THE MANUFACTURER ON THE USE OF ANY OF ITS PRODUCTS.
J. PROVIDE SUBMITTALS FOR CONCRETE MIX DESIGN, REINFORCING, AND ADMIXTURES FOR APPROVAL 14 DAYS (MIN) PRIOR TO CONSTRUCTION.
K. PROVIDE CONCRETE REINFORCING CONFORMING TO THE FOLLOWING:
a. BARS, STIRRUPS AND TIES: ASTM A615 GRADE 60
b. WELDED WIRE REINFORCEMENT: ASTM A1064
L. LAP ALL REBAR A MINIMUM OF 48 BAR DIAMETERS UNLESS NOTED OTHERWISE.
M. THE CONTRACTOR SHALL USE A QUALIFIED AND EXPERIENCED CONSTRUCTION SURVEYOR, TO LAY OUT THE PROPER LOCATION OF ALL EMBEDDED ANCHOR RODS, EMBEDDED CONNECTION PLATES FOR STRUCTURAL STEEL COLUMNS AND BEAMS, AND CORRECT LOCATION AND ELEVATION OF CONCRETE COLUMN DOWELS BEFORE THEY ARE ENCASED IN CONCRETE.
N. FOR SLABS ON GRADE, SAW-CUT SLAB CONTROL JOINTS AT COLUMN LINES SUCH THAT:
a. EACH AREA BOUNDED BY CONTROL JOINTS DOES NOT EXCEED 160 SF
b. THE DISTANCE BETWEEN CONTROL JOINTS DOES NOT EXCEED 14 FEET IN EITHER DIRECTION
c. THE RATIO OF LENGTH TO WIDTH OF ANY AREA BOUNDED BY CONTROL JOINTS DOES NOT EXCEED 2:1.
d. SUBMIT DRAWING OF PROPOSED CONSTRUCTION JOINT LOCATIONS IN CONCRETE FOR SLAB ON GRADE FOR APPROVAL.
O. WHERE THE FLOOR SLAB ABUTS A CMU OR CONCRETE WALL, PROVIDE A BOND BREAK.
P. COMPLY WITH ACI-305 AND ACI-308 FOR HOT AND COLD WEATHER CONSTRUCTION AND FINISHING REQUIREMENTS AS APPLICABLE.

SPECIAL INSPECTIONS

- A. SPECIAL STRUCTURAL TESTS AND INSPECTIONS SHALL BE PERFORMED ON THIS PROJECT IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE (IBC).
B. SPECIAL STRUCTURAL TESTS AND INSPECTIONS SHALL BE PERFORMED BY AN AGENCY SELECTED BY THE OWNER AND APPROVED BY THE ENGINEER OF RECORD. THE AGENCY SHALL MEET ALL OF THE REQUIREMENTS FOR APPROVAL INDICATED IN CHAPTER 17 OF THE IBC.
C. THE CONTRACTOR SHALL COORDINATE THE INSPECTION SERVICES IN ACCORDANCE WITH THE PROGRESS OF THE WORK. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE TO THE INSPECTOR TO ALLOW PROPER SCHEDULING OF PERSONNEL.
D. ALL REPORTS AND SHOP CERTIFICATION OF SPECIAL INSPECTIONS TO BE PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP SHALL BE SUBMITTED TO THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DISTRIBUTING THESE REPORTS TO THE SPECIAL INSPECTOR, THE ARCHITECT, AND THE ENGINEER OF RECORD IN A TIMELY MANNER.
E. SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK. A FINAL REPORT OF INSPECTIONS DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON PRIOR TO THE START OF WORK BY THE APPLICANT AND THE BUILDING OFFICIAL.
F. SPECIAL INSPECTOR SHALL PERIODICALLY INSPECT ROOF DIAPHRAGMS PER IBC SECTION 1705.1.1 AND SHALL PERIODICALLY INSPECT WOOD SHEAR WALLS PER IBC SECTION 1705.12.2.2.

WOOD FRAMING

- A. ALL WOOD FRAMING SHALL CONFORM TO THE REQUIREMENTS OF NDS-2015 "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION WITH SUPPLEMENT".
B. PROVIDE PRESERVATIVE-PRESSURE-TREATED (PT) LUMBER AND PLYWOOD WHERE IN CONTACT WITH CONCRETE OR MASONRY AND WHERE INDICATED.
C. PROVIDE DIMENSIONAL LUMBER CONFORMING TO THE FOLLOWING:
a. CONVENTIONAL FRAMING: # 2 SOUTHERN PINE, KD19
b. WALL STUDS: SPF STUD GRADE (OR BETTER)
c. PSL: PARALLAM PLUS BEAMS, AWP/A USE CATEGORY UC3A, SERVICE LEVEL 2
D. PROVIDE STRUCTURAL PANELS CONFORMING TO THE FOLLOWING:
a. OSB SHEATHING OR ALL-VENEER PLYWOOD PANELS, GROUP 1, AMERICAN PLYWOOD ASSOCIATION (APA) RATED AS FOLLOWS:
1. WALL SHEATHING / BRACING: 1/2", RATED SHEATHING, 32/16, EXPOSURE 1
2. ROOF SHEATHING: 19/32" OR 5/8", RATED SHEATHING, 40/20, EXPOSURE 1
E. FASTENING SCHEDULE: COMPLY WITH IBC TABLE 2304.9.1 UNLESS OTHERWISE INDICATED.

WOOD TRUSSES

- A. ENGINEERED METAL-PLATE-CONNECTED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE SPECIFICATIONS OF THE TRUSS PLATE INSTITUTE (TPI) BY A LICENSED PROFESSIONAL ENGINEER CURRENTLY REGISTERED TO PRACTICE IN THE COMMONWEALTH OF VIRGINIA.
B. THE CONTRACTOR SHALL SUBMIT ENGINEERED TRUSS SHOP DRAWINGS TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL. TRUSS SHOP DRAWINGS SHALL INDICATE ALL LOADING CASES CONSIDERED, MAXIMUM DEFLECTIONS AND MAXIMUM END REACTIONS FOR EACH TYPICAL TRUSS CONFIGURATION. TRUSS SHOP DRAWINGS SHALL INDICATE COMPRESSION MEMBERS REQUIRING ADDITIONAL FIELD-INSTALLED LATERAL BRACING.
C. TRUSS DEFLECTIONS SHALL BE LIMITED TO 1/360 OF SPAN.
D. TRUSS LAYOUT SHOWN ON FRAMING PLAN IS SCHEMATIC. ACTUAL TRUSS LAYOUT SHALL BE AS SPECIFIED BY THE MANUFACTURER AND THE DESIGN SHALL BE CERTIFIED BY MANUFACTURER'S REGISTERED ENGINEER.
E. FABRICATOR DESIGN TRUSSES TO MEET IBC LOADING REQUIREMENTS FOR THE DESIGN LOADING INDICATED. APPLY COEFFICIENTS FOR DRIFT, SLIDE, ROOF CONFIGURATION, AND EXPOSURE.
F. THE FABRICATOR'S ENGINEER IS REQUIRED TO SELECT AND FURNISH ALL HANGERS NECESSARY TO CONNECT ANY TRUSS TO ANOTHER TRUSS (SUCH AS A GIRDER-TRUSS).
G. COMPLY WITH ALL BRACING REQUIREMENTS INDICATED BY THE FABRICATOR, THE TPI SPECIFICATIONS, TPI-BWT, TPI-HIB, AND THESE DRAWINGS.
H. TRUSS MEMBERS: SOUTHERN PINE, KILN-DRIED TO MC OF 19% OR LESS, SIZE AND GRADE AS REQUIRED BY DESIGN, BUT NOT LESS THAN NO. 2 FOR ANY MEMBER.
I. CONNECT BEARING POINTS OF ROOF TRUSSES TO SUPPORTING CONSTRUCTION WITH ANCHORS INDICATED. WHERE ANCHORS ARE NOT DESIGNATED, INSTALL ANCHORS EQUAL TO SIMPSON HI OR H2.5A AT ALL ROOF AND ATTIC TRUSS BEARING POINTS; AND DOUBLE H2.5A ANCHORS AT ALL GIRDER-TRUSS BEARING POINTS.
J. ALL TEMPORARY AND PERMANENT TRUSS BRACING SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES H18-01 PUBLISHED BY THE TRUSS PLATE INSTITUTE. BRACING IS TO BE FASTENED TO EACH CROSSING TRUSS MEMBER WITH A MINIMUM OF TWO 16 D NAILS.
K. TRUSSES SHALL BE HANDLED DURING MANUFACTURING, DELIVERY AND BY THE CONTRACTOR AT THE JOB SITE SO AS NOT TO BE SUBJECTED TO EXCESSIVE BENDING. TRUSSES SHALL BE PROTECTED FROM DAMAGE THAT MIGHT RESULT FROM ON-SITE ACTIVITIES AND ENVIRONMENTAL CONDITIONS. TRUSSES SHALL BE HANDLED IN SUCH A WAY SO AS TO PREVENT TOPPLING WHEN BANDING IS REMOVED.
L. APPARENT DAMAGE TO TRUSSES, IF ANY, SHALL BE REPORTED TO TRUSS MANUFACTURER PRIOR TO ERECTION.
M. TRUSSES SHALL BE SET AND SECURED LEVEL AND PLUMB, AND IN CORRECT LOCATION. EACH TRUSS SHALL BE HELD IN CORRECT ALIGNMENT UNTIL SPECIFIED PERMANENT RESTRAINT AND BRACING IS INSTALLED.
N. CUTTING AND ALTERING OF TRUSSES IS NOT PERMITTED. IF ANY TRUSS SHOULD BECOME BROKEN, DAMAGED, OR ALTERED, REPAIR DESIGNED BY A REGISTERED DESIGN PROFESSIONAL IS REQUIRED.
O. CONCENTRATED LOADS SHALL NOT BE PLACED ON TOP OF TRUSSES UNTIL ALL SPECIFIED RESTRAINT AND BRACING HAS BEEN INSTALLED AND STRUCTURAL SHEATHING IS PERMANENTLY NAILED IN PLACE. SPECIFICALLY AVOID STACKING FULL BUNDLES OF CONSTRUCTION MATERIALS OR OTHER CONCENTRATED LOADS ON TOP OF TRUSSES.



250 E Elizabeth Street Suite 114 Harrisonburg, VA 22802



ELKTON PAVILION

ELKTON, VIRGINIA

Table with 3 columns: No., Description, Date. Empty rows for project notes.

FOR CONSTRUCTION GENERAL NOTES

Project number 22099S Date 11/3/2022 Drawn by TJS Checked by TJS

S0.1

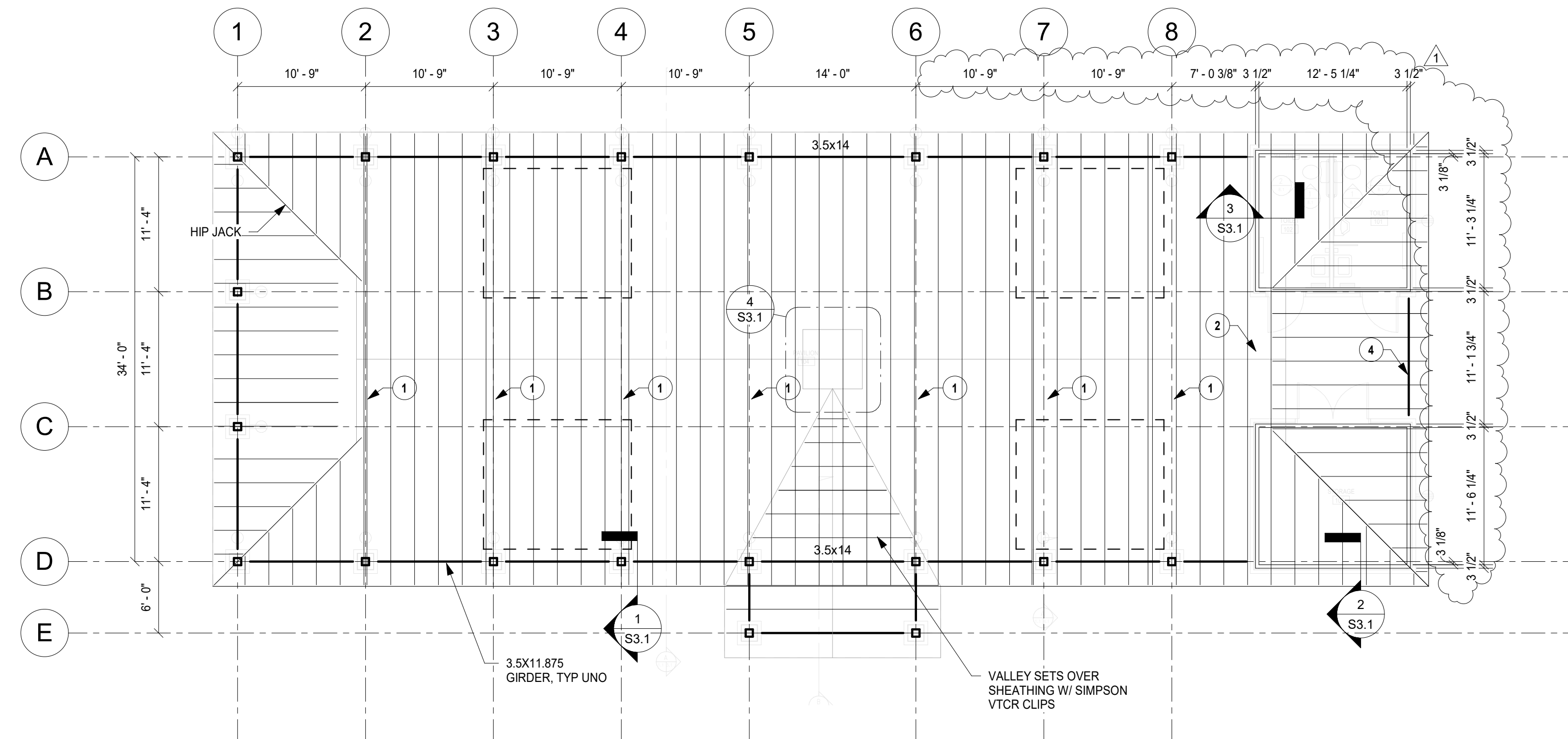
Scale

- FOUNDATION NOTES:**
- SEE S0.01 FOR TYPICAL GENERAL NOTES.
  - TYPICAL SLAB ON GRADE SHALL BE 6" THICK ON 12" VDOT#57 CONSOLIDATED STONE BASE; EXTEND STONE BASE 2' BEYOND SLAB EDGE. REINFORCE SLAB W/ EUCLID PSI FIBERSTRAND F MICRO-FIBER AT 1.5 LB/YD<sup>3</sup> (OR APPROVED EQUAL). INSTALL REINFORCING IN REINFRANT CORNERS PER TYPICAL DETAIL.
  - POSTS SHALL BE PRESSURE TREATED 8X8 GREEN POSTS BY ADVANCED POST SOLUTIONS (OR APPROVED EQUAL).
  - T/PIER = 0'-8" UNO.

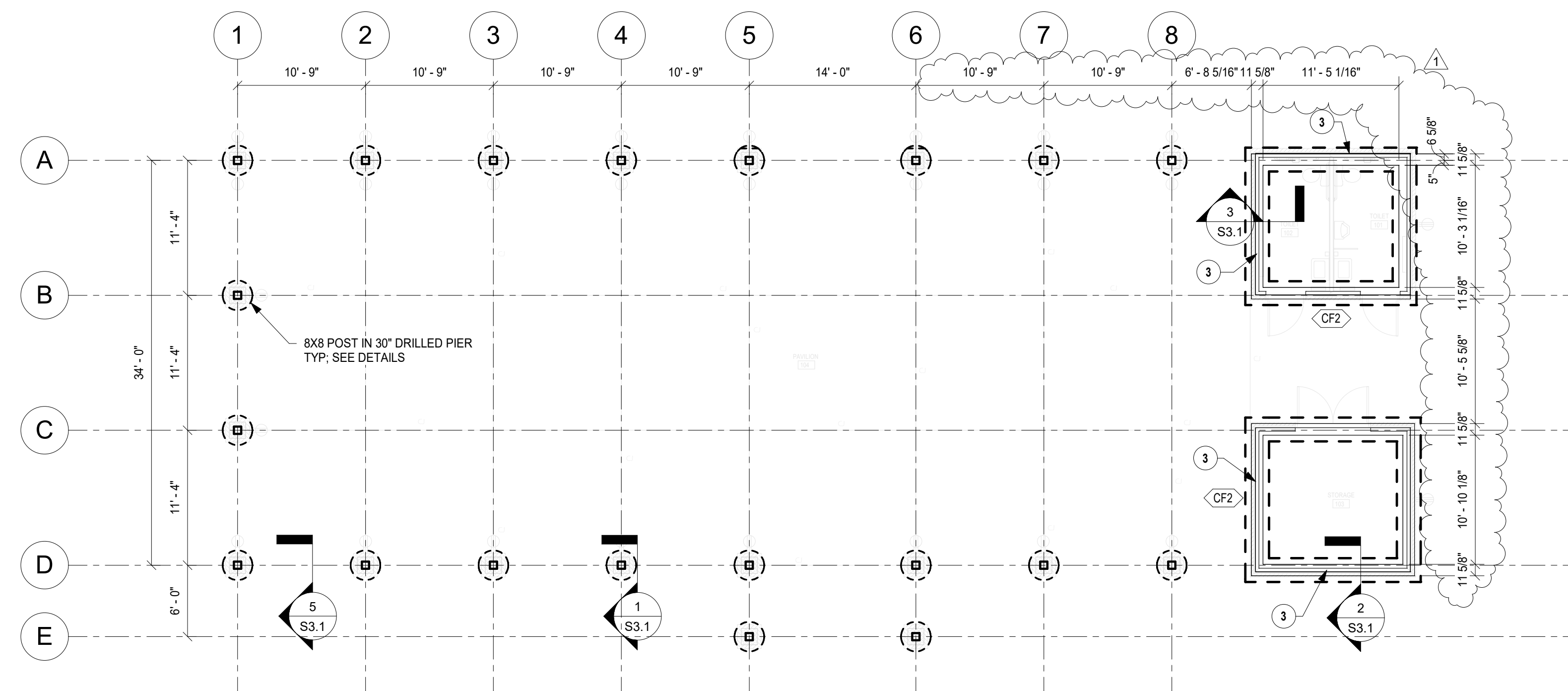
- ROOF FRAMING NOTES:**
- SEE S0.01 FOR TYPICAL GENERAL NOTES.
  - ROOF FRAMING CONSISTS OF 5/8" SHEATHING ON ENGINEERED WOOD TRUSSES AT 24" O-C (MAX). FASTEN SHEATHING TO TRUSSES W/ 8d AT 6" O-C AT EDGES AND 12" O-C IN THE FIELD.
  - GIRDERS SHALL BE 3-1/2"x11-7/8" PARALLAM PLUS PSL BEAMS BY TRUSJOIST (OR EQUAL) UNO. HANG FROM POSTS W/ SIMPSON HUCO412-SDS TYP UNO.
  - STRUCTURAL WORKS SHOWN ON THESE PLANS MUST BE COORDINATED WITH ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL WORK SHOWN ON DRAWINGS PREPARED BY OTHERS. GC SHALL BE RESPONSIBLE FOR COORDINATION OF WORK BY ALL TRADES.
  - TYPICAL DOOR HEADERS ARE 2-2X10 + (2) PLYS 1/2" PLYWOOD W/ 2X4 TOP AND BOTTOM PLATES WITH 2-2X4 JACK STUDS AND 2-2X4 KING STUD. UNO. PROVIDE HEADERS FOR ALL WALL OPENINGS; TYPICAL WINDOW AND DOOR OPENING HEADERS ARE GENERALLY NOT SHOWN ON STRUCTURAL DRAWINGS; REFERENCE ARCHITECTURAL DRAWINGS FOR ALL WALL OPENINGS.
  - GENERAL CONTRACTOR WILL PROVIDE STEEPLE SHOP DRAWING WITH REACTIONS TO TRUSS DESIGNER FOR SUPPORT OF STEEPLE.
  - SEE ARCH DWGS FOR LOUVERS.

CONSTRUCTION KEYNOTES	
KEY MARK	KEYNOTE
1	ALIGN DRAG TRUSS DESIGNED FOR A MIN 184 PLF DRAG LOAD WITH POST. TRUSS SHALL HAVE 2X6 TOP & BOTTOM CHORD MIN.
2	ALIGN DRAG/GABLE TRUSS DESIGNED FOR A MIN 184 PLF DRAG LOAD WITH FRAMED WALLS. TRUSS SHALL HAVE 2X6 TOP & BOTTOM CHORD MIN.
3	SHEAR WALL W/ 7/16" RATED SHEATHING NAILED W/ 8D AT 4" O-C. BLOCK ALL FREE EDGES. PROVIDE 2-2X4 STUDS AT EA WALL END W/ SIMPSON HDJ2-SDS2.5 HOLDOWN W/ 5/8" ANCHOR ROD EMBEDDED 24" IN CMU FDN WALL.
4	DOUBLE JACK+KING STUD AT GIRDER ENDS.

CONTINUOUS FTG SCHEDULE - CFX		
MARK	SIZE (WxT)	REINFORCING
CF2	2' x 12"	(2) #5 CONT



2 ROOF FRAMING PLAN  
S1.0 1/8" = 1'-0"



1 FOUNDATION PLAN  
S1.0 1/8" = 1'-0"

No.	Description	Date
1	ARCH REVS	7/3/2023

FOR  
CONSTRUCTION  
PLANS

Project number	22099S
Date	11/3/2022
Drawn by	TJS
Checked by	TJS

S1.0

Scale As indicated



ELKTON PAVILION

ELKTON, VIRGINIA

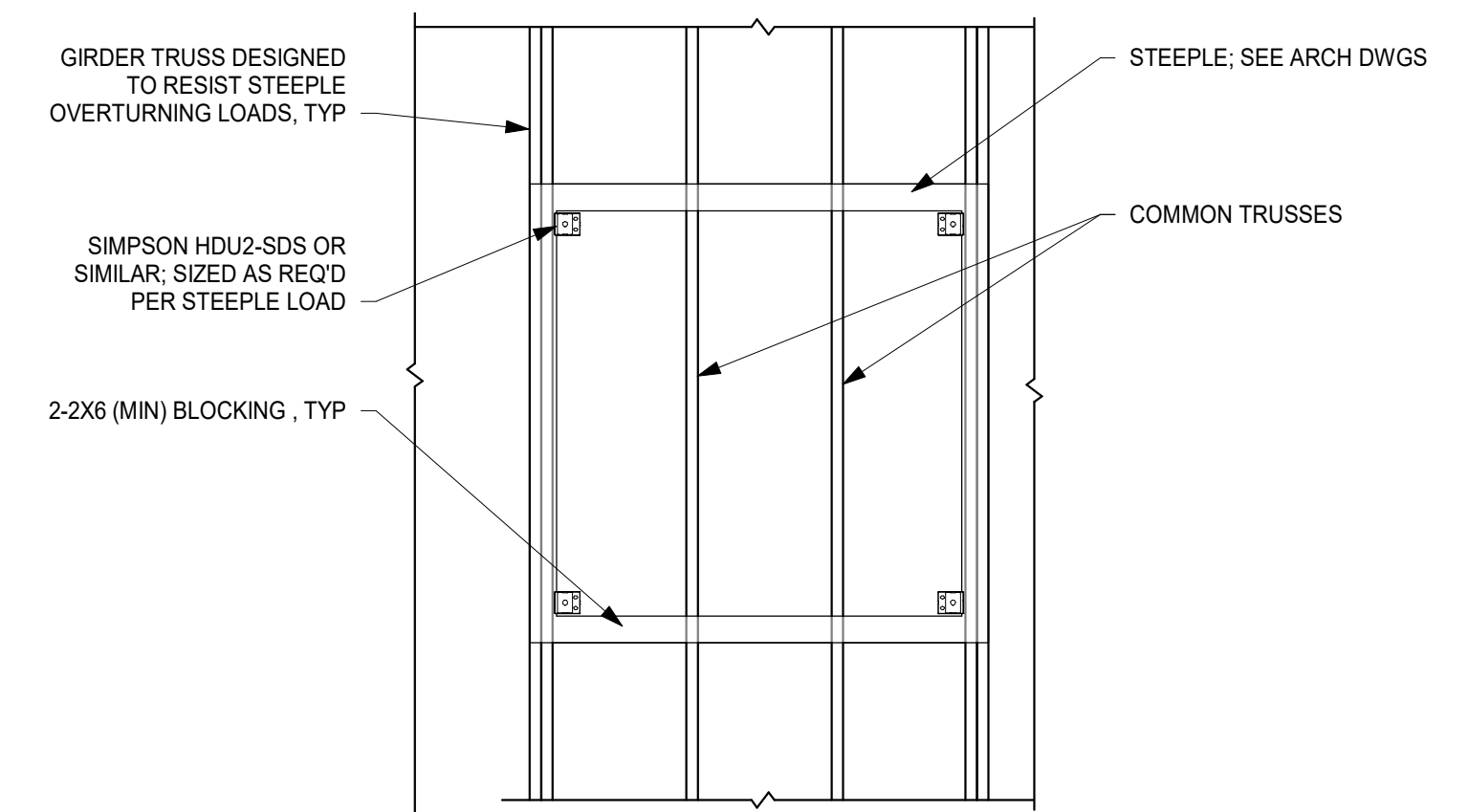
No.	Description	Date

**FOR CONSTRUCTION**  
**SECTIONS AND DETAILS**

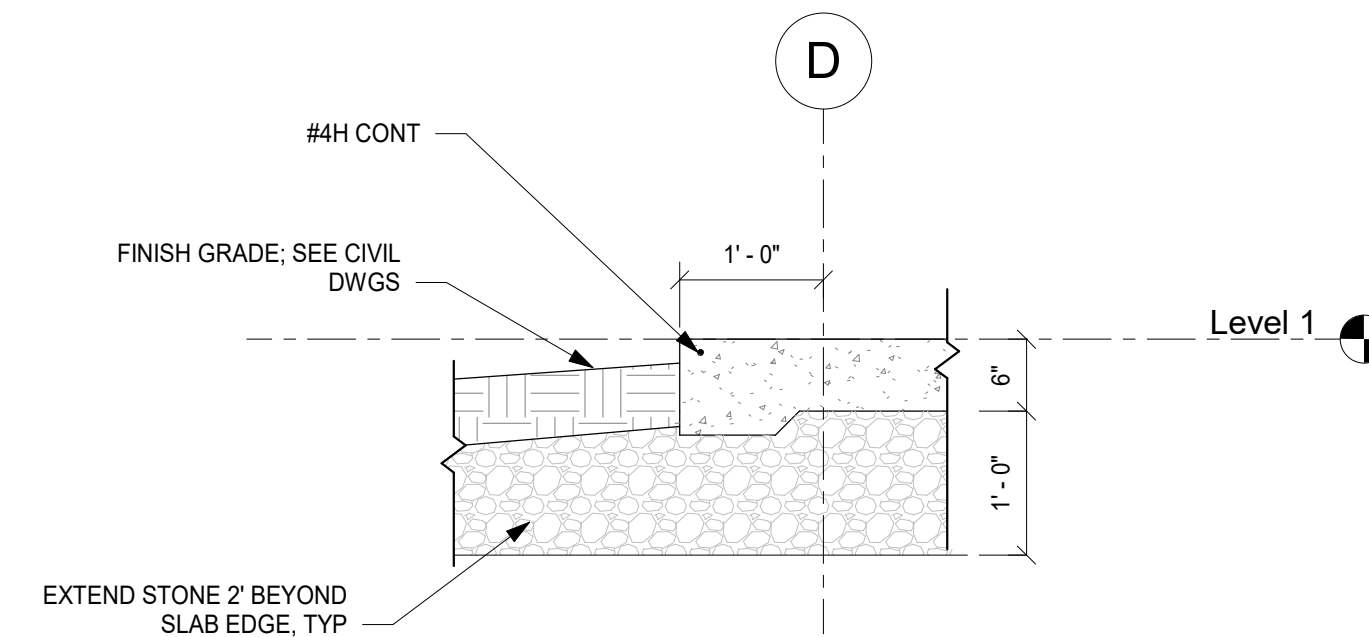
Project number 22099S  
Date 11/3/2022  
Drawn by TJS  
Checked by TJS

**S3.1**

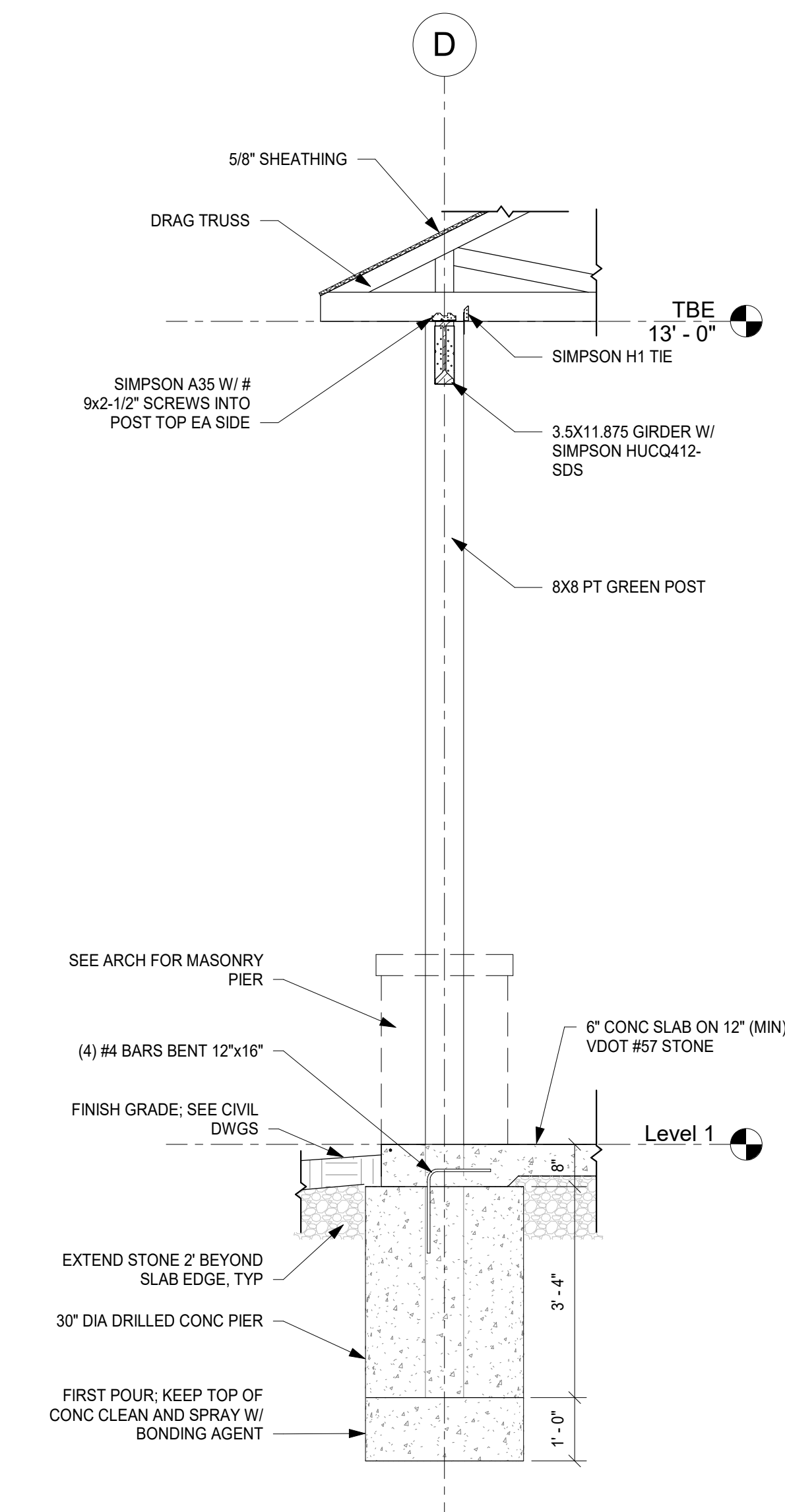
Scale As indicated



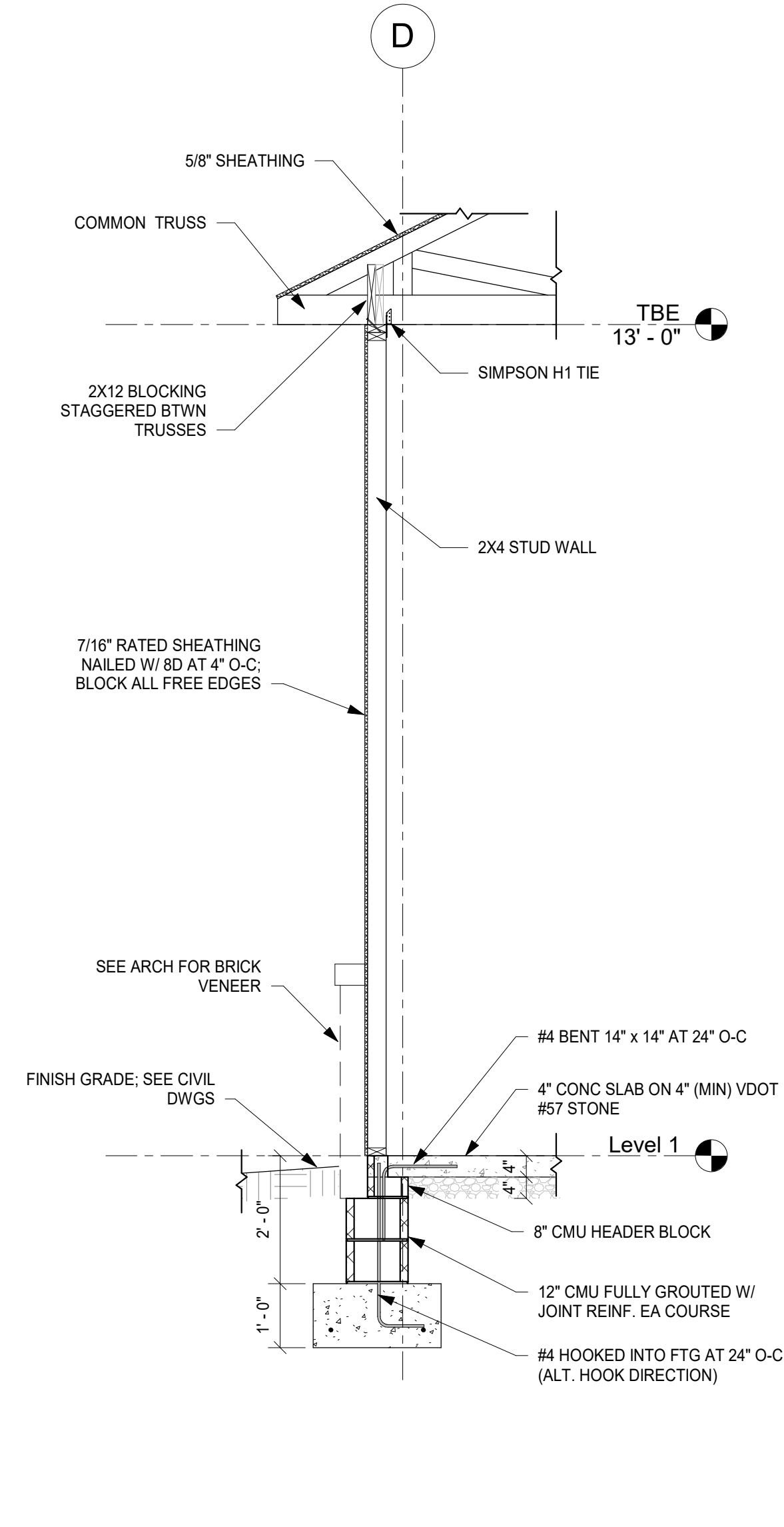
**4** ROOF FRAMING PLAN DETAIL AT STEEPLE  
S1.0 S3.1 1/2" = 1'-0"



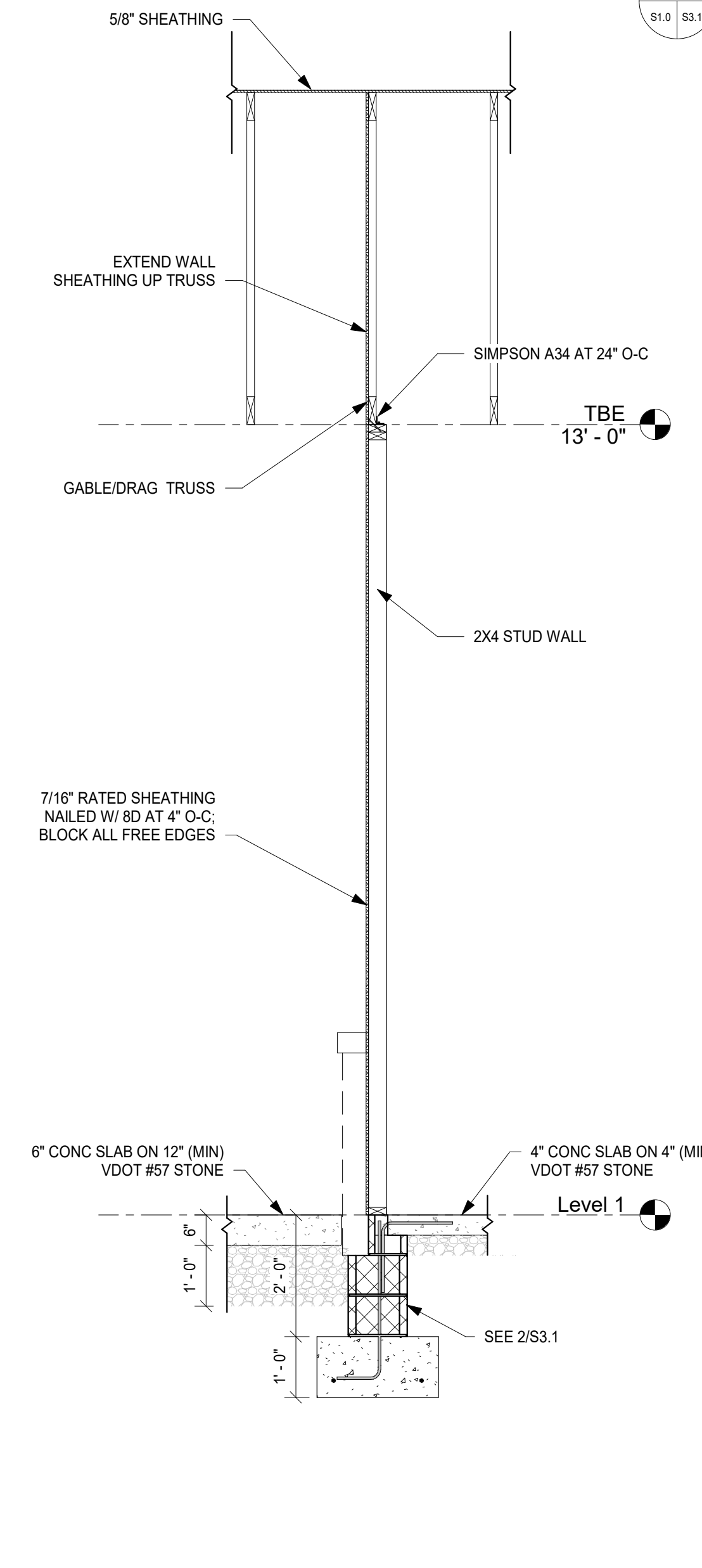
**5** TYPICAL SLAB EDGE TURN-DOWN  
S1.0 S3.1 3/4" = 1'-0"



**1** SECTION AT POST  
S1.0 S3.1 1/2" = 1'-0"

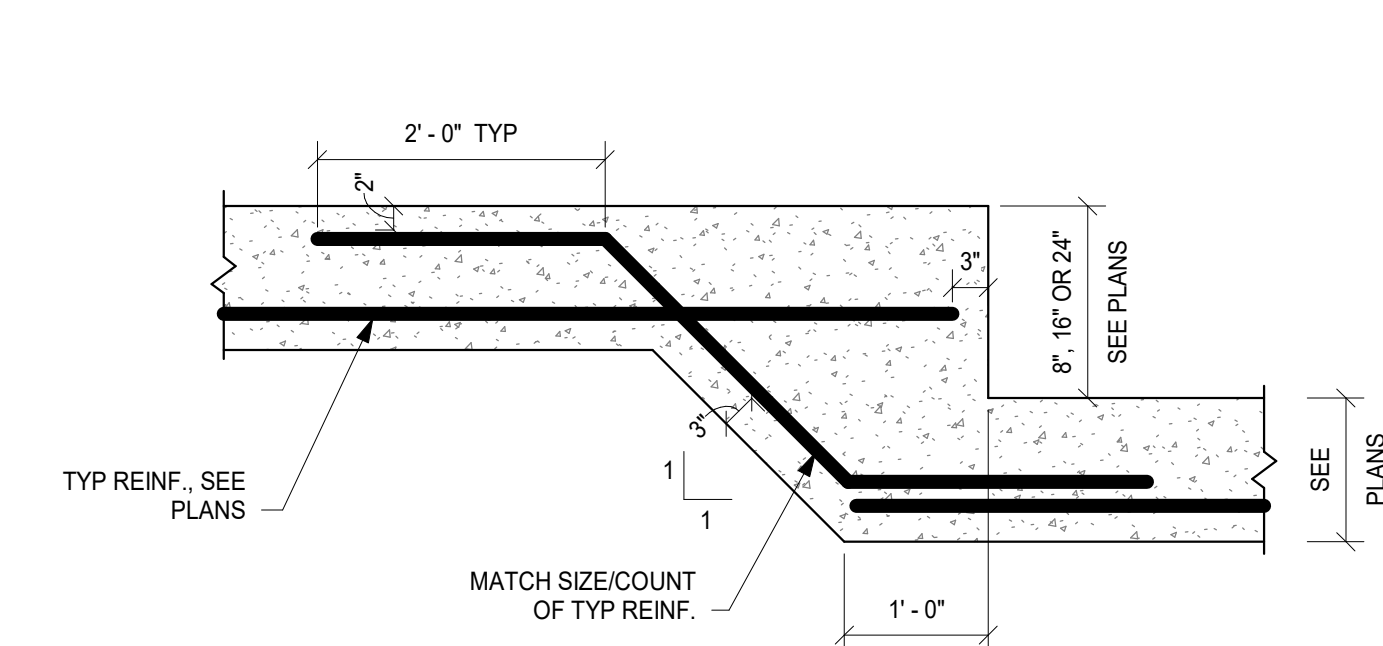


**2** SECTION AT EXTERIOR WALL  
S1.0 S3.1 1/2" = 1'-0"

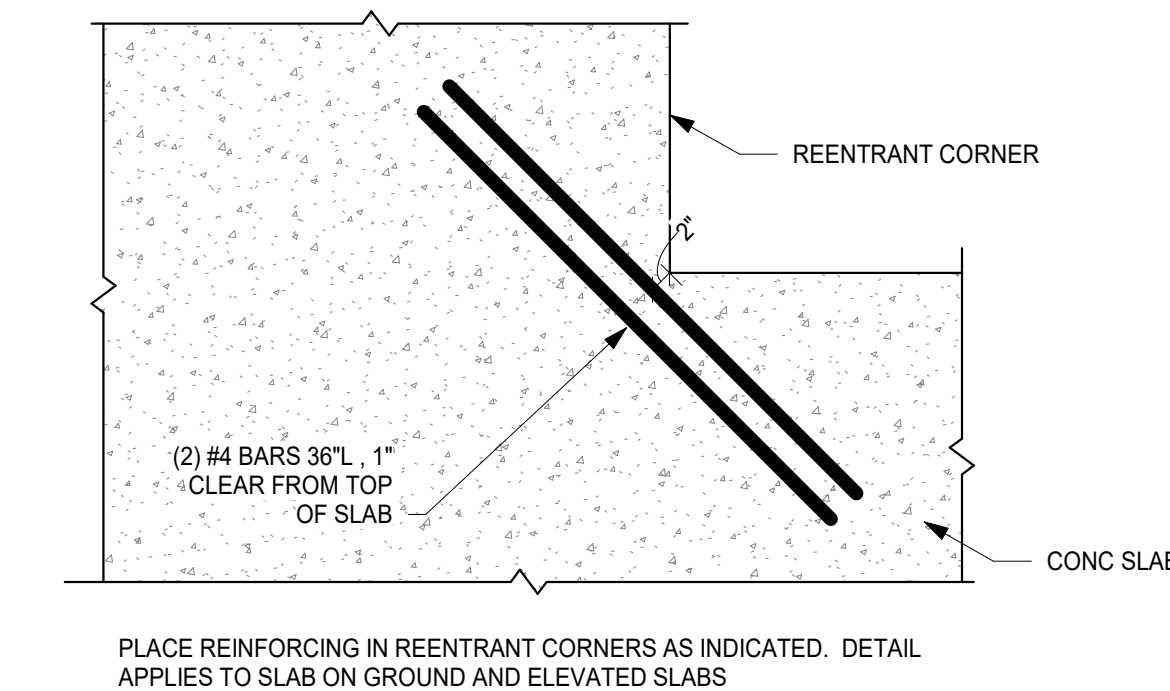
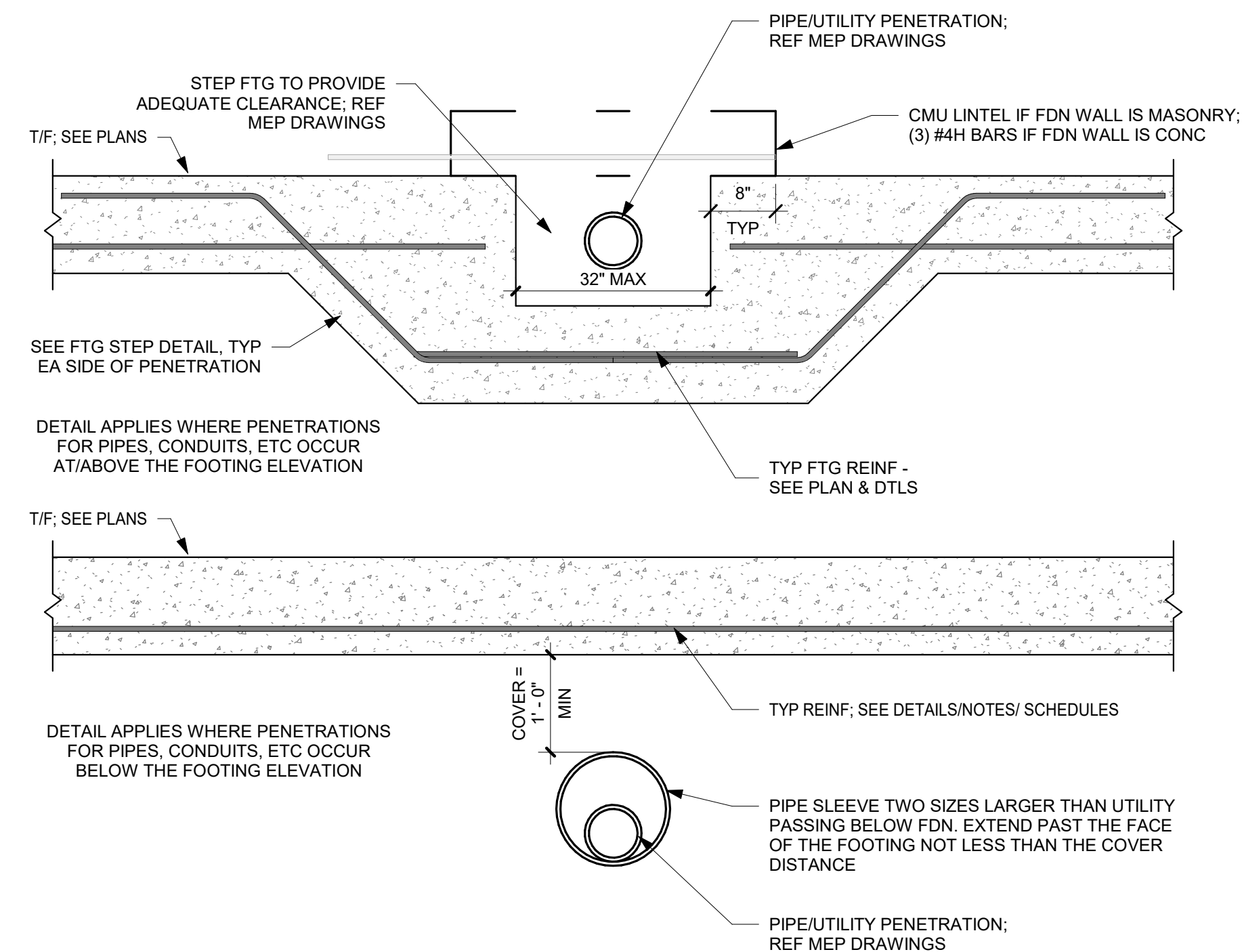


**3** SECTION AT INTERIOR WALL  
S1.0 S3.1 1/2" = 1'-0"

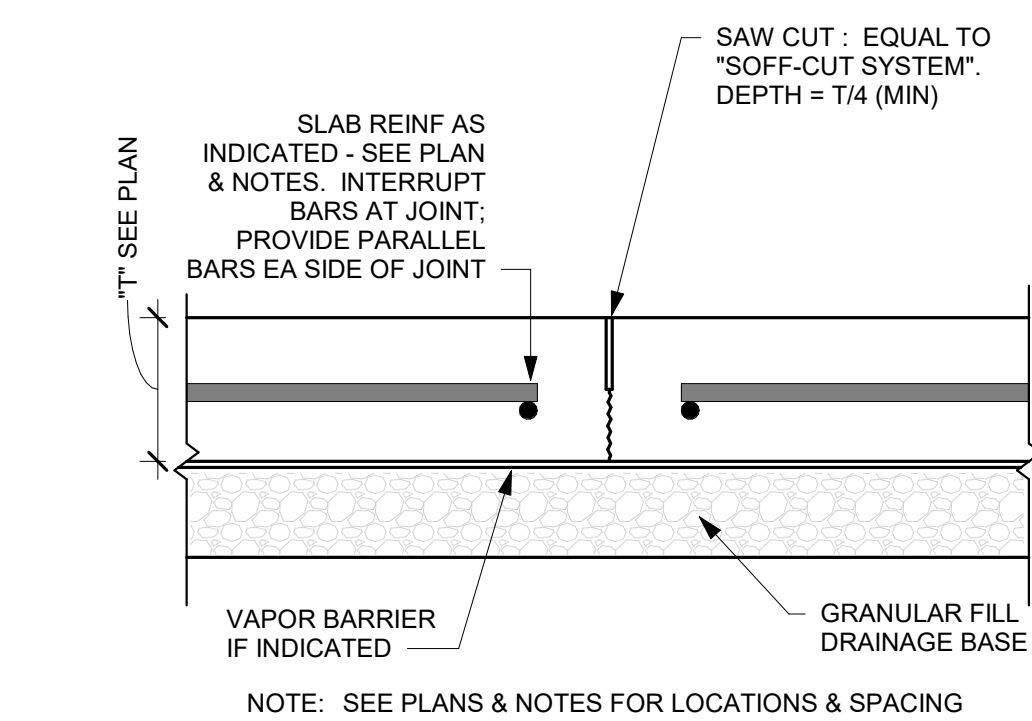
**CONC-FOOTING STEP DETAIL**  
NO SCALE



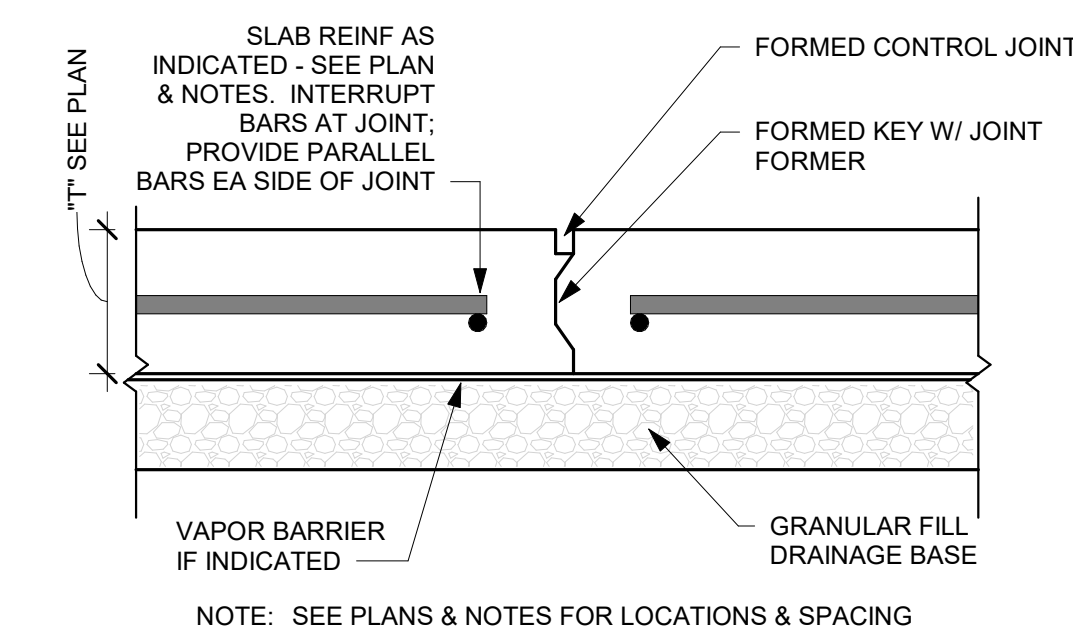
**CONC-FTG AT UTILITY CROSSING**  
NO SCALE



**CONC-REENTRANT CORNER DETAIL**  
NO SCALE



**CONC-SAWN SLAB CONTROL JOINT**  
NO SCALE



**CONC-CONSTRUCTION JOINT**  
NO SCALE